

3: Auburn Ravine Road & Bowman Road  
 HCM Signalized Intersection Capacity Analysis

Buildout Without Forest Ranch - Mitig.  
 PM Peak Hour

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↖	↑	↗	↖	↕					↖	↑	↗
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0					4.0	4.0	4.0
Lane Util. Factor	1.00	1.00	1.00	1.00	0.95					1.00	1.00	1.00
Frbp, ped/bikes	1.00	1.00	0.97	1.00	0.98					1.00	1.00	0.98
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00					1.00	1.00	1.00
Frt	1.00	1.00	0.85	1.00	0.92					1.00	1.00	0.85
Flt Protected	0.95	1.00	1.00	0.95	1.00					0.95	1.00	1.00
Satd. Flow (prot)	1770	1863	1530	1770	3217					1770	1863	1551
Flt Permitted	0.95	1.00	1.00	0.95	1.00					0.95	1.00	1.00
Satd. Flow (perm)	1770	1863	1530	1770	3217					1770	1863	1551
Volume (vph)	125	533	240	316	652	659	0	0	0	348	155	65
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	136	579	261	343	709	716	0	0	0	378	168	71
RTOR Reduction (vph)	0	0	154	0	192	0	0	0	0	0	0	54
Lane Group Flow (vph)	136	579	107	343	1233	0	0	0	0	378	168	17
Conf. Peds. (#/hr)	5		5	5		5	5		5	5		5
Turn Type	Prot		Perm	Prot						Split		Perm
Protected Phases	7	4		3	8					6	6	
Permitted Phases			4									6
Actuated Green, G (s)	10.3	37.9	37.9	20.9	48.5					22.2	22.2	22.2
Effective Green, g (s)	9.3	36.9	36.9	19.9	47.5					21.2	21.2	21.2
Actuated g/C Ratio	0.10	0.41	0.41	0.22	0.53					0.24	0.24	0.24
Clearance Time (s)	3.0	3.0	3.0	3.0	3.0					3.0	3.0	3.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0					3.0	3.0	3.0
Lane Grp Cap (vph)	183	764	627	391	1698					417	439	365
v/s Ratio Prot	0.08	0.31		c0.19	c0.44					c0.21	0.09	
v/s Ratio Perm			0.17									0.05
v/c Ratio	0.74	0.76	0.17	0.88	0.73					0.91	0.38	0.05
Uniform Delay, d1	39.2	22.7	16.8	33.9	16.3					33.4	28.9	26.6
Progression Factor	1.00	1.00	1.00	1.09	0.99					1.00	1.00	1.00
Incremental Delay, d2	15.0	6.9	0.6	17.7	2.5					22.8	0.6	0.1
Delay (s)	54.2	29.7	17.4	54.5	18.6					56.2	29.5	26.6
Level of Service	D	C	B	D	B					E	C	C
Approach Delay (s)		29.8			25.5			0.0			45.5	
Approach LOS		C			C			A			D	

Intersection Summary			
HCM Average Control Delay	30.4	HCM Level of Service	C
HCM Volume to Capacity ratio	0.85		
Actuated Cycle Length (s)	90.0	Sum of lost time (s)	8.0
Intersection Capacity Utilization	75.7%	ICU Level of Service	D
Analysis Period (min)	15		

c Critical Lane Group

6: Auburn Ravine Road & I-80 Westbound Off-Ramp  
 HCM Signalized Intersection Capacity Analysis

Buildout Without Forest Ranch - Mitig.  
 PM Peak Hour

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↑	↑↑↑		↑	↑
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)		4.0	4.0		4.0	4.0
Lane Util. Factor		1.00	0.91		1.00	1.00
Frbp, ped/bikes		1.00	1.00		1.00	0.98
Flpb, ped/bikes		1.00	1.00		1.00	1.00
Frt		1.00	1.00		1.00	0.85
Flt Protected		1.00	1.00		0.95	1.00
Satd. Flow (prot)		1863	5085		1770	1551
Flt Permitted		1.00	1.00		0.95	1.00
Satd. Flow (perm)		1863	5085		1770	1551
Volume (vph)	0	876	1427	0	373	200
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	0	952	1551	0	405	217
RTOR Reduction (vph)	0	0	0	0	0	0
Lane Group Flow (vph)	0	952	1551	0	405	217
Confl. Peds. (#/hr)	5			5	5	5
Turn Type						Perm
Protected Phases		4	8		6	
Permitted Phases						6
Actuated Green, G (s)		59.0	59.0		24.0	24.0
Effective Green, g (s)		58.0	58.0		24.0	24.0
Actuated g/C Ratio		0.64	0.64		0.27	0.27
Clearance Time (s)		3.0	3.0		4.0	4.0
Vehicle Extension (s)		3.0	3.0		3.0	3.0
Lane Grp Cap (vph)		1201	3277		472	414
v/s Ratio Prot		c0.51	0.30		c0.23	
v/s Ratio Perm						0.14
v/c Ratio		0.79	0.47		0.86	0.52
Uniform Delay, d1		11.6	8.2		31.4	28.1
Progression Factor		0.24	0.29		1.00	1.00
Incremental Delay, d2		3.3	0.3		14.3	1.2
Delay (s)		6.1	2.6		45.7	29.3
Level of Service		A	A		D	C
Approach Delay (s)		6.1	2.6		40.0	
Approach LOS		A	A		D	
<b>Intersection Summary</b>						
HCM Average Control Delay			11.1		HCM Level of Service	B
HCM Volume to Capacity ratio			0.81			
Actuated Cycle Length (s)			90.0		Sum of lost time (s)	8.0
Intersection Capacity Utilization			126.6%		ICU Level of Service	H
Analysis Period (min)			15			

c Critical Lane Group

8: Auburn Ravine Road & I-80 Eastbound Ramps  
 HCM Signalized Intersection Capacity Analysis

Buildout Without Forest Ranch - Mitig.  
 PM Peak Hour

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0			4.0	4.0		4.0	4.0			
Lane Util. Factor	1.00	1.00			0.95	1.00		1.00	0.88			
Frbp, ped/bikes	1.00	1.00			1.00	0.97		1.00	0.97			
Flpb, ped/bikes	1.00	1.00			1.00	1.00		0.99	1.00			
Frt	1.00	1.00			1.00	0.85		1.00	0.85			
Flt Protected	0.95	1.00			1.00	1.00		0.95	1.00			
Satd. Flow (prot)	1770	1863			3539	1530		1762	2693			
Flt Permitted	0.95	1.00			1.00	1.00		0.95	1.00			
Satd. Flow (perm)	1770	1863			3539	1530		1762	2693			
Volume (vph)	250	954	0	0	1067	521	360	10	844	0	0	0
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	272	1037	0	0	1160	566	391	11	917	0	0	0
RTOR Reduction (vph)	0	0	0	0	0	148	0	0	58	0	0	0
Lane Group Flow (vph)	272	1037	0	0	1160	418	0	402	859	0	0	0
Confl. Peds. (#/hr)	5		5	5		5	5		5	5		5
Turn Type	Prot					Perm	Perm		Perm			
Protected Phases	7	4			8			2				
Permitted Phases						8	2		2			
Actuated Green, G (s)	13.8	50.8			34.0	34.0		33.2	33.2			
Effective Green, g (s)	12.8	49.8			33.0	33.0		32.2	32.2			
Actuated g/C Ratio	0.14	0.55			0.37	0.37		0.36	0.36			
Clearance Time (s)	3.0	3.0			3.0	3.0		3.0	3.0			
Vehicle Extension (s)	3.0	3.0			3.0	3.0		3.0	3.0			
Lane Grp Cap (vph)	252	1031			1298	561		630	963			
v/s Ratio Prot	0.15	c0.56			0.33							
v/s Ratio Perm						0.37		0.23	0.34			
v/c Ratio	1.08	1.01			0.89	0.74		0.64	0.89			
Uniform Delay, d1	38.6	20.1			26.8	24.8		24.1	27.3			
Progression Factor	0.96	0.93			1.16	1.51		1.00	1.00			
Incremental Delay, d2	66.7	23.0			1.0	0.8		2.1	10.5			
Delay (s)	103.9	41.7			32.2	38.5		26.2	37.7			
Level of Service	F	D			C	D		C	D			
Approach Delay (s)		54.6			34.2			34.2			0.0	
Approach LOS		D			C			C			A	

Intersection Summary			
HCM Average Control Delay	40.4	HCM Level of Service	D
HCM Volume to Capacity ratio	0.98		
Actuated Cycle Length (s)	90.0	Sum of lost time (s)	8.0
Intersection Capacity Utilization	126.6%	ICU Level of Service	H
Analysis Period (min)	15		

c Critical Lane Group

11: Auburn Ravine Road & Lincoln Way  
 HCM Signalized Intersection Capacity Analysis

Buildout Without Forest Ranch - Mitig.  
 PM Peak Hour

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0		4.0	4.0	4.0
Lane Util. Factor	0.97	1.00	1.00	1.00	0.95	1.00	0.97	0.95		1.00	0.95	1.00
Frbp, ped/bikes	1.00	1.00	0.98	1.00	1.00	0.98	1.00	0.98		1.00	1.00	0.98
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		1.00	1.00	1.00
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	0.92		1.00	1.00	0.85
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00		0.95	1.00	1.00
Satd. Flow (prot)	3433	1863	1551	1770	3539	1551	3433	3186		1770	3539	1551
Flt Permitted	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00		0.95	1.00	1.00
Satd. Flow (perm)	3433	1863	1551	1770	3539	1551	3433	3186		1770	3539	1551
Volume (vph)	420	1048	310	203	843	345	490	345	424	282	195	255
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	457	1139	337	221	916	375	533	375	461	307	212	277
RTOR Reduction (vph)	0	0	71	0	0	218	0	246	0	0	0	0
Lane Group Flow (vph)	457	1139	266	221	916	157	533	590	0	307	212	277
Conf. Peds. (#/hr)	5		5	5		5	5		5	5		5
Turn Type	Split		Perm	Split		Perm	Prot			Prot		Perm
Protected Phases	4	4		8	8		5	2		1	6	
Permitted Phases			4			8						6
Actuated Green, G (s)	31.0	31.0	31.0	19.0	19.0	19.0	16.0	16.0		11.0	11.0	11.0
Effective Green, g (s)	31.0	31.0	31.0	18.0	18.0	18.0	15.0	15.0		10.0	10.0	10.0
Actuated g/C Ratio	0.34	0.34	0.34	0.20	0.20	0.20	0.17	0.17		0.11	0.11	0.11
Clearance Time (s)	4.0	4.0	4.0	3.0	3.0	3.0	3.0	3.0		3.0	3.0	3.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0		3.0	3.0	3.0
Lane Grp Cap (vph)	1182	642	534	354	708	310	572	531		197	393	172
v/s Ratio Prot	0.13	c0.61		0.12	c0.26		0.16	c0.26		c0.17	0.06	
v/s Ratio Perm			0.22			0.24						0.18
v/c Ratio	0.39	1.77	0.50	0.62	1.29	0.51	0.93	1.11		1.56	0.54	1.61
Uniform Delay, d1	22.3	29.5	23.3	32.9	36.0	32.1	37.0	37.5		40.0	37.8	40.0
Progression Factor	1.02	1.00	1.02	1.00	1.00	1.00	1.00	1.00		1.00	1.00	1.00
Incremental Delay, d2	0.4	350.7	1.2	3.4	142.6	1.3	22.2	73.3		274.6	1.4	300.0
Delay (s)	23.0	380.3	25.1	36.3	178.6	33.4	59.2	110.8		314.6	39.3	340.0
Level of Service	C	F	C	D	F	C	E	F		F	D	F
Approach Delay (s)		233.9			121.8			90.7			250.1	
Approach LOS		F			F			F			F	

Intersection Summary			
HCM Average Control Delay	171.0	HCM Level of Service	F
HCM Volume to Capacity ratio	1.59		
Actuated Cycle Length (s)	90.0	Sum of lost time (s)	16.0
Intersection Capacity Utilization	118.8%	ICU Level of Service	H
Analysis Period (min)	15		

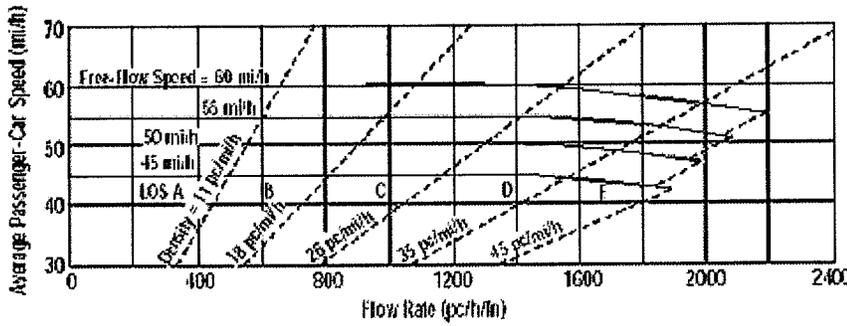
c Critical Lane Group

**APPENDIX I**

**CUMULATIVE + PROJECT CONDITIONS  
BUILDOUT "WITHOUT FOREST RANCH" SCENARIO  
MODIFIED LEVEL OF SERVICE CRITERIA  
LEVEL OF SERVICE CALCULATION WORKSHEETS**



**MULTILANE HIGHWAYS WORKSHEET(Direction 1)**



Application	Input	Output
Operational (LOS)	FFS, N, $v_p$	LOS, S, D
Design (N)	FFS, LOS, $v_p$	N, S, D
Design ( $v_p$ )	FFS, LOS, N	$v_p$ , S, D
Planning (LOS)	FFS, N, AADT	LOS, S, D
Planning (N)	FFS, LOS, AADT	N, S, D
Planning ( $v_p$ )	FFS, LOS, N	$v_p$ , S, D

General Information		Site Information	
Analyst	MKL	Highway/Direction to Travel	Foresthill Road - Mitigated
Agency or Company	MRO Engineers, Inc.	From/To	Bridge to Spring Garden Rd
Date Performed	4/3/2007	Jurisdiction	Placer County
Analysis Time Period	AM Peak Hour	Analysis Year	Buildout Without Forest Ranch

Project Description Foresthill Divide Community Plan

Oper.(LOS)
  Des. (N)
  Plan. ( $v_p$ )

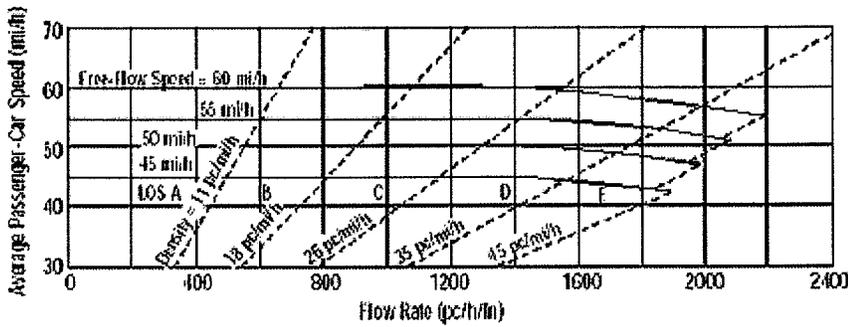
Flow Inputs			
Volume, V (veh/h)	1040	Peak-Hour Factor, PHF	0.88
AADT(veh/h)		%Trucks and Buses, $P_T$	6
Peak-Hour Prop of AADT (veh/d)		%RVs, $P_R$	2
Peak-Hour Direction Prop, D		General Terrain:	Grade
DDHV (veh/h)		Grade Length (mi)	4.00
Driver Type Adjustment	1.00	Up/Down %	3.00
		Number of Lanes	2

Calculate Flow Adjustments			
$f_p$	1.00	$E_R$	3.0
$E_T$	2.5	$f_{HV}$	0.885

Speed Inputs		Calc Speed Adj and FFS	
Lane Width, LW (ft)	12.0	$f_{LW}$ (mi/h)	0.0
Total Lateral Clearance, LC (ft)	12.0	$f_{LC}$ (mi/h)	0.0
Access Points, A (A/mi)	2	$f_A$ (mi/h)	0.5
Median Type, M	Undivided	$f_M$ (mi/h)	1.6
FFS (measured)		FFS (mi/h)	57.9
Base Free-Flow Speed, BFFS	60.0		

Operations		Design	
Operational (LOS)		Design (N)	
Flow Rate, $v_p$ (pc/h/ln)	667	Required Number of Lanes, N	
Speed, S (mi/h)	57.9	Flow Rate, $v_p$ (pc/h)	
D (pc/mi/ln)	11.5	Max Service Flow Rate (pc/h/ln)	
LOS	B	Design LOS	

**MULTILANE HIGHWAYS WORKSHEET(Direction 2)**



Application	Input	Output
Operational (LOS)	FFS, N, $v_p$	LOS, S, D
Design (N)	FFS, LOS, $v_p$	N, S, D
Design ( $v_p$ )	FFS, LOS, N	$v_p$ , S, D
Planning (LOS)	FFS, N, AADT	LOS, S, D
Planning (N)	FFS, LOS, AADT	N, S, D
Planning ( $v_p$ )	FFS, LOS, N	$v_p$ , S, D

General Information	Site Information
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Analyst: MKL Agency or Company: MRO Engineers, Inc. Date Performed: 4/3/2007 Analysis Time Period: AM Peak Hour	Highway/Direction to Travel: Foresthill Road - Mitigated From/To: Bridge to Spring Garden Rd Jurisdiction: Placer County Analysis Year: Buildout Without Forest Ranch
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Project Description: Foresthill Divide Community Plan

Oper.(LOS)                     
  Des. (N)                                     
  Plan. ( $v_p$ )

**Flow Inputs**

Volume, V (veh/h)	1768	Peak-Hour Factor, PHF	0.88
AAADT(veh/h)		%Trucks and Buses, $P_T$	2
Peak-Hour Prop of AAADT (veh/d)		%RVs, $P_R$	2
Peak-Hour Direction Prop, D		General Terrain:	Grade
DDHV (veh/h)		Grade Length (mi)	4.00
Driver Type Adjustment	1.00	Up/Down %	-3.00
		Number of Lanes	2

**Calculate Flow Adjustments**

$f_p$	1.00	$E_R$	1.2
$E_T$	1.5	$f_{HV}$	0.986

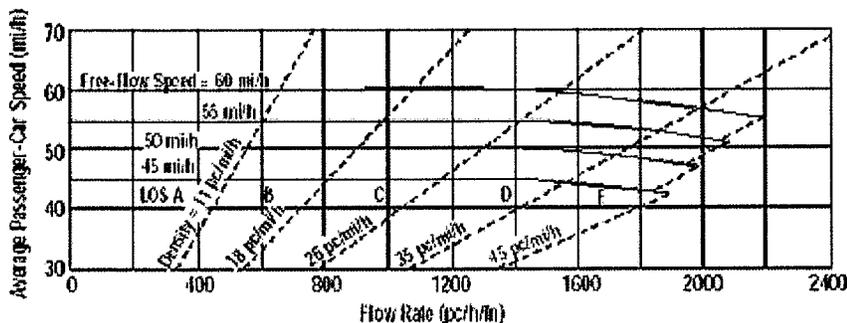
Speed Inputs	Calc Speed Adj and FFS
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Lane Width, LW (ft)	12.0	$f_{LW}$ (mi/h)	0.0
Total Lateral Clearance, LC (ft)	12.0	$f_{LC}$ (mi/h)	0.0
Access Points, A (A/mi)	2	$f_A$ (mi/h)	0.5
Median Type, M	Undivided	$f_M$ (mi/h)	1.6
FFS (measured)		FFS (mi/h)	57.9
Base Free-Flow Speed, BFFS	60.0		

Operations	Design
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Operational (LOS) Flow Rate, $v_p$ (pc/h/ln)                      1018 Speed, S (mi/h)                                      57.9 D (pc/mi/ln)    17.6 LOS    B	Design (N) Required Number of Lanes, N Flow Rate, $v_p$ (pc/h) Max Service Flow Rate (pc/h/ln) Design LOS
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**MULTILANE HIGHWAYS WORKSHEET(Direction 1)**



Application	Input	Output
Operational (LOS)	FFS, N, $v_p$	LOS, S, D
Design (N)	FFS, LOS, $v_p$	N, S, D
Design ( $v_p$ )	FFS, LOS, N	$v_p$ , S, D
Planning (LOS)	FFS, N, AADT	LOS, S, D
Planning (N)	FFS, LOS, AADT	N, S, D
Planning ( $v_p$ )	FFS, LOS, N	$v_p$ , S, D

**General Information**

Analyst: MKL  
 Agency or Company: MRO Engineers, Inc.  
 Date Performed: 4/3/2007  
 Analysis Time Period: AM Peak Hour

**Site Information**

Highway/Direction to Travel: Foresthill Road - Mitigated  
 From/To: Spring Garden to Todd Valley W  
 Jurisdiction: Placer County  
 Analysis Year: Buildout Without Forest Ranch

Project Description: Foresthill Divide Community Plan

Oper.(LOS)
  Des. (N)
  Plan. (vp)

**Flow Inputs**

Volume, V (veh/h)	1160	Peak-Hour Factor, PHF	0.75
AAADT(veh/h)		%Trucks and Buses, $P_T$	3
Peak-Hour Prop of AAADT (veh/d)		%RVs, $P_R$	2
Peak-Hour Direction Prop, D		General Terrain:	Grade
DDHV (veh/h)		Grade Length (mi)	1.70
Driver Type Adjustment	1.00	Up/Down %	3.40
		Number of Lanes	2

**Calculate Flow Adjustments**

$f_p$	1.00	$E_R$	3.0
$E_T$	3.8	$f_{HV}$	0.891

**Speed Inputs**

Lane Width, LW (ft)	12.0
Total Lateral Clearance, LC (ft)	12.0
Access Points, A (A/mi)	4
Median Type, M	Undivided
FFS (measured)	
Base Free-Flow Speed, BFFS	60.0

**Calc Speed Adj and FFS**

$f_{LW}$ (mi/h)	0.0
$f_{LC}$ (mi/h)	0.0
$f_A$ (mi/h)	1.0
$f_M$ (mi/h)	1.6
FFS (mi/h)	57.4

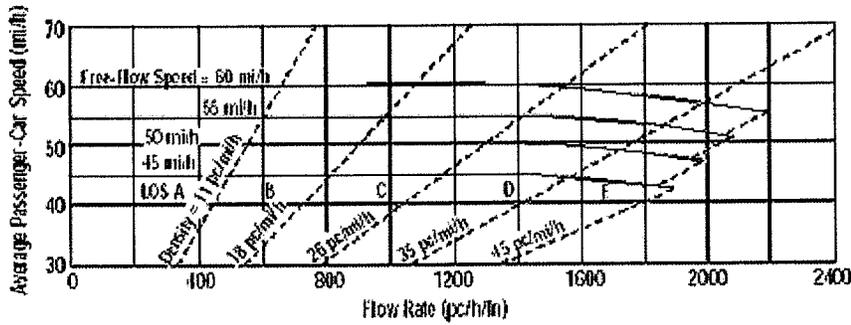
**Operations**

Operational (LOS)	
Flow Rate, $v_p$ (pc/h/ln)	868
Speed, S (mi/h)	57.4
D (pc/mi/ln)	15.1
LOS	B

**Design**

Design (N)	
Required Number of Lanes, N	
Flow Rate, $v_p$ (pc/h)	
Max Service Flow Rate (pc/h/ln)	
Design LOS	

## MULTILANE HIGHWAYS WORKSHEET(Direction 2)



Application	Input	Output
Operational (LOS)	FFS, N, $v_p$	LOS, S, D
Design (N)	FFS, LOS, $v_p$	N, S, D
Design ( $v_p$ )	FFS, LOS, N	$v_p$ , S, D
Planning (LOS)	FFS, N, AADT	LOS, S, D
Planning (N)	FFS, LOS, AADT	N, S, D
Planning ( $v_p$ )	FFS, LOS, N	$v_p$ , S, D

General Information	Site Information
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Analyst Agency or Company Date Performed Analysis Time Period	MKL MRO Engineers, Inc. 4/3/2007 AM Peak Hour
Highway/Direction to Travel From/To Jurisdiction Analysis Year	Foresthill Road - Mitigated Spring Garden to Todd Valley W Placer County Buildout Without Forest Ranch

Project Description Foresthill Divide Community Plan

Oper.(LOS)
  Des. (N)
  Plan. (vp)

### Flow Inputs

Volume, V (veh/h)	1215	Peak-Hour Factor, PHF	0.88
AADT(veh/h)		%Trucks and Buses, $P_T$	2
Peak-Hour Prop of AADT (veh/d)		%RVs, $P_R$	2
Peak-Hour Direction Prop, D		General Terrain:	Grade
DDHV (veh/h)		Grade Length (mi)	1.70
Driver Type Adjustment	1.00	Up/Down %	-3.40
		Number of Lanes	2

### Calculate Flow Adjustments

$f_p$	1.00	$E_R$	1.2
$E_T$	1.5	$f_{HV}$	0.986

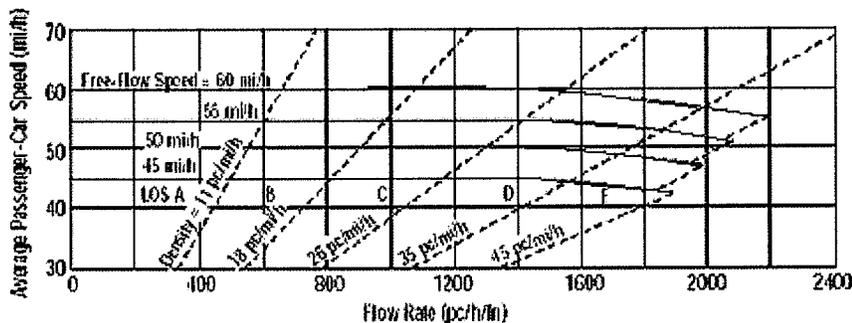
Speed Inputs	Calc Speed Adj and FFS
--------------	------------------------

Lane Width, LW (ft)	12.0	$f_{LW}$ (mi/h)	0.0
Total Lateral Clearance, LC (ft)	12.0	$f_{LC}$ (mi/h)	0.0
Access Points, A (A/mi)	5	$f_A$ (mi/h)	1.3
Median Type, M	Undivided	$f_M$ (mi/h)	1.6
FFS (measured)		FFS (mi/h)	57.2
Base Free-Flow Speed, BFFS	60.0		

Operations	Design
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Operational (LOS)	Design (N)
Flow Rate, $v_p$ (pc/h/ln)	Required Number of Lanes, N
Speed, S (mi/h)	Flow Rate, $v_p$ (pc/h)
D (pc/mi/ln)	Max Service Flow Rate (pc/h/ln)
LOS	Design LOS

## MULTILANE HIGHWAYS WORKSHEET(Direction 1)



Application	Input	Output
Operational (LOS)	FFS, N, $v_p$	LOS, S, D
Design (N)	FFS, LOS, $v_p$	N, S, D
Design ( $v_p$ )	FFS, LOS, N	$v_p$ , S, D
Planning (LOS)	FFS, N, AADT	LOS, S, D
Planning (N)	FFS, LOS, AADT	N, S, D
Planning ( $v_p$ )	FFS, LOS, N	$v_p$ , S, D

### General Information

Analyst: MKL  
 Agency or Company: MRO Engineers, Inc.  
 Date Performed: 4/3/2007  
 Analysis Time Period: AM Peak Hour

### Site Information

Highway/Direction to Travel: Foresthill Road - Mitigated  
 From/To: Todd Valley W to Owl Hill Ct  
 Jurisdiction: Placer County  
 Analysis Year: Buildout Without Forest Ranch

Project Description: Foresthill Divide Community Plan

Oper.(LOS)

Des. (N)

Plan. ( $v_p$ )

### Flow Inputs

Volume, V (veh/h)	1049	Peak-Hour Factor, PHF	0.59
AADT(veh/h)		%Trucks and Buses, $P_T$	2
Peak-Hour Prop of AADT (veh/d)		%RVs, $P_R$	2
Peak-Hour Direction Prop, D		General Terrain:	Grade
DDHV (veh/h)		Grade Length (mi)	1.20
Driver Type Adjustment	1.00	Up/Down %	3.00
		Number of Lanes	2

### Calculate Flow Adjustments

$f_p$	1.00	$E_R$	3.0
$E_T$	2.5	$f_{HV}$	0.935

### Speed Inputs

Lane Width, LW (ft): 12.0  
 Total Lateral Clearance, LC (ft): 12.0  
 Access Points, A (A/mi): 3  
 Median Type, M: Undivided  
 FFS (measured):  
 Base Free-Flow Speed, BFFS: 60.0

### Calc Speed Adj and FFS

$f_{LW}$  (mi/h): 0.0  
 $f_{LC}$  (mi/h): 0.0  
 $f_A$  (mi/h): 0.8  
 $f_M$  (mi/h): 1.6  
 FFS (mi/h): 57.7

### Operations

Operational (LOS):  
 Flow Rate,  $v_p$  (pc/h/ln): 951  
 Speed, S (mi/h): 57.7  
 D (pc/mi/ln): 16.5  
 LOS: B

### Design

Design (N):  
 Required Number of Lanes, N:  
 Flow Rate,  $v_p$  (pc/h):  
 Max Service Flow Rate (pc/h/ln):  
 Design LOS:

## DIRECTIONAL TWO-LANE HIGHWAY SEGMENT WORKSHEET

General Information		Site Information	
Analyst	MKL	Highway / Direction of Travel	Foresthill Road - WB - Mitig.
Agency or Company	MRO Engineers, Inc.	From/To	Todd Valley W to Owl Hill Ct.
Date Performed	2/27/07	Jurisdiction	Placer County
Analysis Time Period	AM Peak Hour	Analysis Year	Buildout Without Forest Ranch

Project Description: *Foresthill Divide Community Plan*

### Input Data

Segment length,  $L_1$  \_\_\_\_\_ mi

Class I highway
  Class II highway

Terrain
  Level
  Rolling

Grade Length 1.20 mi Up/down -3.0

Peak-hour factor, PHF 0.79

No-passing zone 100%

% Trucks and Buses,  $P_T$  2%

% Recreational vehicles,  $P_R$  2%

Access points/ mi 4

Analysis direction vol.,  $V_d$  1060veh/h  
 Opposing direction vol.,  $V_o$  1049veh/h

### Average Travel Speed

	Analysis Direction (d)	Opposing Direction (o)
Passenger-car equivalents for trucks, $E_T$ (Exhibit 20-9 or 20-15)	1.1	3.7
Passenger-car equivalents for RVs, $E_R$ (Exhibit 20-9 or 20-17)	1.0	1.0
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$	0.998	0.948
Grade adjustment factor <sup>1</sup> , $f_G$ (Exhibit 20-7 or 20-13)	1.00	1.00
Directional flow rate <sup>2</sup> , $v_i(\text{pc/h})=V_i/(\text{PHF} \cdot f_{HV} \cdot f_G)$	1344	1401
Free-Flow Speed from Field Measurement	Estimated Free-Flow Speed	
Field measured speed <sup>3</sup> , $S_{FM}$ mi/h		60.0 mi/h
Observed volume <sup>3</sup> , $V_f$ veh/h		0.0 mi/h
Free-flow speed, $\text{FFS}_d = S_{FM} + 0.007776(V_f / f_{HV})$ mi/h		1.0 mi/h
Adjustment for no-passing zones, $f_{np}$ (Exhibit 20-19) 0.9 mi/h		59.0 mi/h
		Average travel speed, $\text{ATS} = \text{FFS}_d - 0.007776 v_p - f_{np}$ 36.8 mi/h

### Percent Time-Spent-Following

	Analysis Direction (d)	Opposing Direction (o)
Passenger-car equivalents for trucks, $E_T$ (Exhibit 20-10 or 20-16)	1.0	1.0
Passenger-car equivalents for RVs, $E_R$ (Exhibit 20-10 or 20-16)	1.0	1.0
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$	1.000	1.000
Grade adjustment factor <sup>1</sup> , $f_G$ (Exhibit 20-8 or 20-14)	1.00	0.93
Directional flow rate <sup>2</sup> , $v_i(\text{pc/h})=V_i/(\text{PHF} \cdot f_{HV} \cdot f_G)$	1342	1422
Base percent time-spent-following <sup>4</sup> , $\text{BPTSF}(\%)=100(1-e^{-a v_d^b})$		88.4
Adj. for no-passing zone, $f_{np}$ (Exhibit 20-20)		13.1
Percent time-spent-following, $\text{PTSF}(\%)=\text{BPTSF}+f_{np}$		94.8

### Level of Service and Other Performance Measures

Level of service, LOS (Exhibit 20-3 or 20-4)	E
Volume to capacity ratio, $v/c=V_p/1,700$	0.79
Peak 15-min veh-miles of travel, $\text{VMT}_{15}(\text{veh-mi})=0.25L_1(V/\text{PHF})$	403
Peak-hour vehicle-miles of travel, $\text{VMT}_{60}(\text{veh-mi})=V \cdot L_1$	1272
Peak 15-min total travel time, $\text{TT}_{15}(\text{veh-h})=\text{VMT}_{15}/\text{ATS}$	11.0

### Notes

1. If the highway is extended segment (level) or rolling terrain,  $f_G=1.0$ .
2. If  $v_i(v_d \text{ or } v_o) \geq 1,700 \text{ pc/h}$ , terminate analysis--the LOS is F.
3. For the analysis direction only.
4. Exhibit 20-21 provides factors a and b.
5. Use alternative Equation 20-14 if some trucks operate at crawl speeds on a specific downgrade.

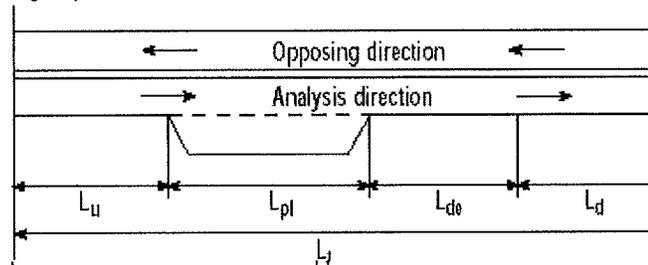
## DIRECTIONAL TWO-LANE HIGHWAY SEGMENT WORKSHEET WITH PASSING LANE WORKSHEET

General Information		Site Information	
Analyst	MKL	Highway of Travel	Foresthill Road - WB - Mitig.
Agency or Company	MRO Engineers, Inc.	From/To	Todd Valley W to Owl Hill Ct.
Date Performed	2/27/07	Jurisdiction	Placer County
Analysis Time Period	AM Peak Hour	Analysis Year	Buildout Without Forest Ranch

Project Description: *Foresthill Divide Community Plan*

### Input Data

Class I highway     Class II highway



Total length of analysis segment, $L_t$ (mi)	1.2
Length of two-lane highway upstream of the passing lane, $L_u$ (mi)	0.2
Length of passing lane including tapers, $L_{pl}$ (mi)	0.9
Average travel speed, $ATS_d$ (from Directional Two-Lane Highway Segment Worksheet)	36.8
Percent time-spent-following, $PTSF_d$ (from Directional Two-Lane Highway Segment Worksheet)	94.8
Level of service <sup>1</sup> , $LOS_d$ (from Directional Two-Lane Highway Segment Worksheet)	E

### Average Travel Speed

Downstream length of two-lane highway within effective length of passing lane for average travel speed, $L_{de}$ (m) (Exhibit 20-23)	1.70
Length of two-lane highway downstream of effective length of the passing lane for avg travel speed, $L_d$ (m) $L_d = L_t - (L_u + L_{pl} + L_{de})$	-1.60
Adj. factor for the effect of passing lane on average speed, $f_{pl}$ (Exhibit 20-24)	1.11
Average travel speed including passing lane <sup>2</sup> , $ATS_{pl} = (ATS_d * L_t) / (L_u + L_d + (L_{pl}/f_{pl}) + (2L_{de}/(1+f_{pl})))$	40.1

### Percent Time-Spent-Following

Downstream length of two-lane highway within effective length of passing lane for percent time-spent-following, $L_{de}$ (m) (Exhibit 20-23)	3.60
Length of two-lane highway downstream of effective length of the passing lane for percent-time-following, $L_d$ (m) $L_d = L_t - (L_u + L_{pl} + L_{de})$	-3.50
Adj. factor for the effect of passing lane on percent time-spent-following, $f_{pl}$ (Exhibit 20-24)	0.62
Percent time-spent-following including passing lane <sup>3</sup> , $PTSF_{pl}$ (%) $PTSF_{pl} = PTSF_d [L_u + L_d + f_{pl}L_{pl} + ((1+f_{pl})/2)L_{de}] / L_t$	64.8

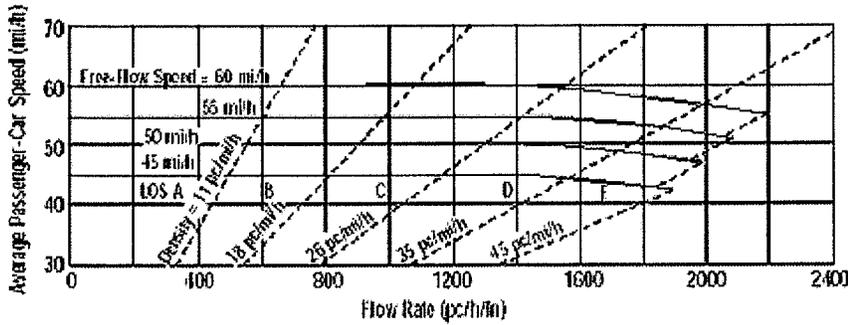
### Level of Service and Other Performance Measures<sup>4</sup>

Level of service including passing lane $LOS_{pl}$ (Exhibit 20-3 or 20-4)	D
Peak 15-min total travel time, $TT_{15}$ (veh-h) $TT_{15} = VMT_{15}/ATS_{pl}$	10.0

### Notes

1. If  $LOS_d = F$ , passing lane analysis cannot be performed.
2. If  $L_d < 0$ , use alternative Equation 20-22.
3. If  $L_d < 0$ , use alternative Equation 20-20.
4. v/c,  $VMT_{15}$  and  $VMT_{60}$  are calculated on Directional Two-Lane Highway Segment Worksheet.

**MULTILANE HIGHWAYS WORKSHEET(Direction 1)**



Application	Input	Output
Operational (LOS)	FFS, $v_p$	LOS, S, D
Design (N)	FFS, LOS, $v_p$	N, S, D
Design ( $v_p$ )	FFS, LOS, N	$v_p$ , S, D
Planning (LOS)	FFS, N, AADT	LOS, S, D
Planning (N)	FFS, LOS, AADT	N, S, D
Planning ( $v_p$ )	FFS, LOS, N	$v_p$ , S, D

**General Information** **Site Information**

Analyst	MKL	Highway/Direction to Travel	Foresthill Road - Mitigated
Agency or Company	MRO Engineers, Inc.	From/To	Bridge to Spring Garden Rd
Date Performed	4/3/2007	Jurisdiction	Placer County
Analysis Time Period	PM Peak Hour	Analysis Year	Buildout Without Forest Ranch

Project Description Foresthill Divide Community Plan

Oper.(LOS)  Des. (N)  Plan. (vp)

**Flow Inputs**

Volume, V (veh/h)	1574	Peak-Hour Factor, PHF	0.88
AAADT(veh/h)		%Trucks and Buses, $P_T$	1
Peak-Hour Prop of AAADT (veh/d)		%RVs, $P_R$	2
Peak-Hour Direction Prop, D		General Terrain:	Grade
DDHV (veh/h)		Grade Length (mi)	4.00
Driver Type Adjustment	1.00	Up/Down %	3.00
		Number of Lanes	2

**Calculate Flow Adjustments**

$f_p$	1.00	$E_R$	3.0
$E_T$	3.0	$f_{HV}$	0.943

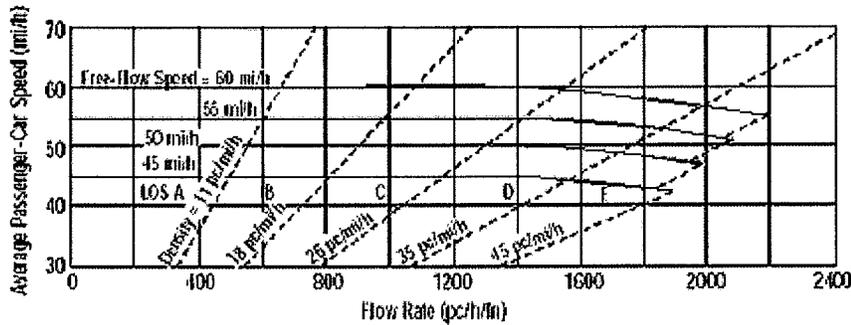
**Speed Inputs** **Calc Speed Adj and FFS**

Lane Width, LW (ft)	12.0	$f_{LW}$ (mi/h)	0.0
Total Lateral Clearance, LC (ft)	12.0	$f_{LC}$ (mi/h)	0.0
Access Points, A (A/mi)	2	$f_A$ (mi/h)	0.5
Median Type, M	Undivided	$f_M$ (mi/h)	1.6
FFS (measured)		FFS (mi/h)	57.9
Base Free-Flow Speed, BFFS	60.0		

**Operations** **Design**

Operational (LOS)		Design (N)	
Flow Rate, $v_p$ (pc/h/ln)	947	Required Number of Lanes, N	
Speed, S (mi/h)	57.9	Flow Rate, $v_p$ (pc/h)	
D (pc/mi/ln)	16.4	Max Service Flow Rate (pc/h/ln)	
LOS	B	Design LOS	

## MULTILANE HIGHWAYS WORKSHEET(Direction 2)



Application	Input	Output
Operational (LOS)	FFS, N, $v_p$	LOS, S, D
Design (N)	FFS, LOS, $v_p$	N, S, D
Design ( $v_p$ )	FFS, LOS, N	$v_p$ , S, D
Planning (LOS)	FFS, N, AADT	LOS, S, D
Planning (N)	FFS, LOS, AADT	N, S, D
Planning ( $v_p$ )	FFS, LOS, N	$v_p$ , S, D

General Information	Site Information
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Analyst Agency or Company Date Performed Analysis Time Period	MKL MRO Engineers, Inc. 4/3/2007 PM Peak Hour
Highway/Direction to Travel From/To Jurisdiction Analysis Year	Foresthill Road - Mitigated Bridge to Spring Garden Rd Placer County Buildout Without Forest Ranch

Project Description Foresthill Divide Community Plan

Oper.(LOS)
  Des. (N)
  Plan. (vp)

### Flow Inputs

Volume, V (veh/h)	1210	Peak-Hour Factor, PHF	0.76
AADT(veh/h)		%Trucks and Buses, $P_T$	4
Peak-Hour Prop of AADT (veh/d)		%RVs, $P_R$	2
Peak-Hour Direction Prop, D		General Terrain:	Grade
DDHV (veh/h)		Grade Length (mi)	4.00
Driver Type Adjustment	1.00	Up/Down %	-3.00
		Number of Lanes	2

### Calculate Flow Adjustments

$f_p$	1.00	$E_R$	1.2
$E_T$	1.5	$f_{HV}$	0.977

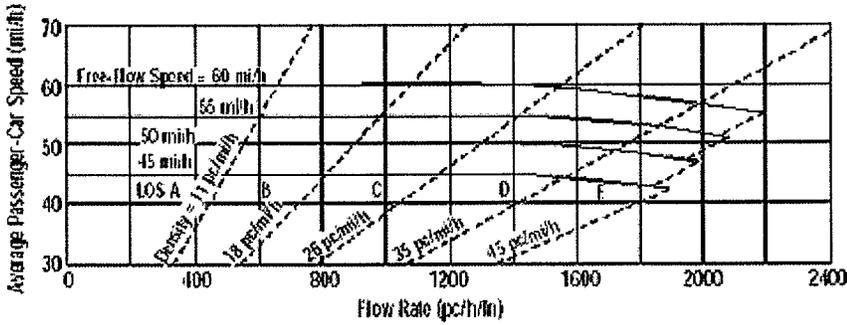
Speed Inputs	Calc Speed Adj and FFS
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Lane Width, LW (ft)	12.0	$f_{LW}$ (mi/h)	0.0
Total Lateral Clearance, LC (ft)	12.0	$f_{LC}$ (mi/h)	0.0
Access Points, A (A/mi)	2	$f_A$ (mi/h)	0.5
Median Type, M	Undivided	$f_M$ (mi/h)	1.6
FFS (measured)		FFS (mi/h)	57.9
Base Free-Flow Speed, BFFS	60.0		

Operations	Design
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Operational (LOS)	Design (N)
Flow Rate, $v_p$ (pc/h/ln)	815
Speed, S (mi/h)	57.9
D (pc/mi/ln)	14.1
LOS	B
	Design (N)
	Required Number of Lanes, N
	Flow Rate, $v_p$ (pc/h)
	Max Service Flow Rate (pc/h/ln)
	Design LOS

**MULTILANE HIGHWAYS WORKSHEET(Direction 1)**



Application	Input	Output
Operational (LOS)	FFS, N, $v_p$	LOS, S, D
Design (N)	FFS, LOS, $v_p$	N, S, D
Design ( $v_p$ )	FFS, LOS, N	$v_p$ , S, D
Planning (LOS)	FFS, N, AADT	LOS, S, D
Planning (N)	FFS, LOS, AADT	N, S, D
Planning ( $v_p$ )	FFS, LOS, N	$v_p$ , S, D

General Information	Site Information
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Analyst: MKL	Highway/Direction to Travel: Foresthill Road - Mitigated
Agency or Company: MRO Engineers, Inc.	From/To: Spring Garden to Todd Valley W
Date Performed: 4/3/2007	Jurisdiction: Placer County
Analysis Time Period: PM Peak Hour	Analysis Year: Buildout Without Forest Ranch

Project Description: Foresthill Divide Community Plan

Oper.(LOS)                     
  Des. (N)                                     
  Plan. ( $v_p$ )

**Flow Inputs**

Volume, V (veh/h)	1117	Peak-Hour Factor, PHF	0.88
AAADT(veh/h)		%Trucks and Buses, $P_T$	2
Peak-Hour Prop of AAADT (veh/d)		%RVs, $P_R$	2
Peak-Hour Direction Prop, D		General Terrain:	Grade
DDHV (veh/h)		Grade Length (mi)	1.70
Driver Type Adjustment	1.00	Up/Down %	3.40
		Number of Lanes	2

**Calculate Flow Adjustments**

$f_p$	1.00	$E_R$	3.0
$E_T$	4.0	$f_{HV}$	0.909

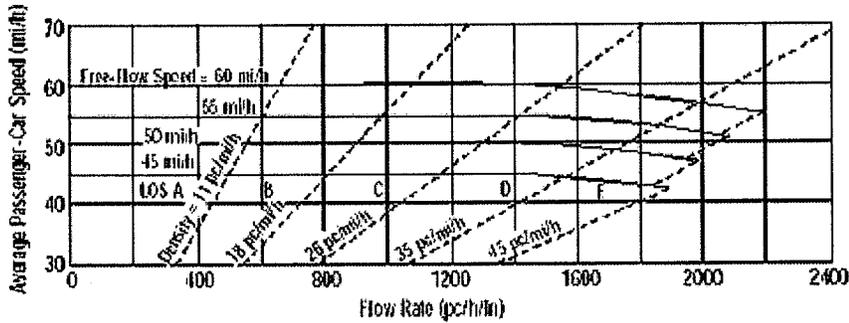
Speed Inputs	Calc Speed Adj and FFS
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Lane Width, LW (ft)	12.0	$f_{LW}$ (mi/h)	0.0
Total Lateral Clearance, LC (ft)	12.0	$f_{LC}$ (mi/h)	0.0
Access Points, A (A/mi)	4	$f_A$ (mi/h)	1.0
Median Type, M	Undivided	$f_M$ (mi/h)	1.6
FFS (measured)		FFS (mi/h)	57.4
Base Free-Flow Speed, BFFS	60.0		

Operations	Design
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Operational (LOS)	Design (N)
Flow Rate, $v_p$ (pc/h/ln)	Required Number of Lanes, N
Speed, S (mi/h)	Flow Rate, $v_p$ (pc/h)
D (pc/mi/ln)	Max Service Flow Rate (pc/h/ln)
LOS	Design LOS

## MULTILANE HIGHWAYS WORKSHEET(Direction 2)



Application	Input	Output
Operational (LOS)	FFS, N, $v_p$	LOS, S, D
Design (N)	FFS, LOS, $v_p$	N, S, D
Design ( $v_p$ )	FFS, LOS, N	$v_p$ , S, D
Planning (LOS)	FFS, N, AADT	LOS, S, D
Planning (N)	FFS, LOS, AADT	N, S, D
Planning ( $v_p$ )	FFS, LOS, N	$v_p$ , S, D

### General Information

Analyst: MKL  
 Agency or Company: MRO Engineers, Inc.  
 Date Performed: 4/3/2007  
 Analysis Time Period: PM Peak Hour

### Site Information

Highway/Direction to Travel: Foresthill Road - Mitigated  
 From/To: Spring Garden to Todd Valley W  
 Jurisdiction: Placer County  
 Analysis Year: Buildout Without Forest Ranch

Project Description: Foresthill Divide Community Plan

Oper.(LOS)

Des. (N)

Plan. (vp)

### Flow Inputs

Volume, V (veh/h)	1305	Peak-Hour Factor, PHF	0.88
AADT(veh/h)		%Trucks and Buses, $P_T$	3
Peak-Hour Prop of AADT (veh/d)		%RVs, $P_R$	2
Peak-Hour Direction Prop, D		General Terrain:	Grade
DDHV (veh/h)		Grade Length (mi)	1.70
Driver Type Adjustment	1.00	Up/Down %	-3.40
		Number of Lanes	2

### Calculate Flow Adjustments

$f_p$	1.00	$E_R$	1.2
$E_T$	1.5	$f_{HV}$	0.981

### Speed Inputs

Lane Width, LW (ft): 12.0  
 Total Lateral Clearance, LC (ft): 12.0  
 Access Points, A (A/mi): 5  
 Median Type, M: Undivided  
 FFS (measured):  
 Base Free-Flow Speed, BFFS: 60.0

### Calc Speed Adj and FFS

$f_{LW}$  (mi/h): 0.0  
 $f_{LC}$  (mi/h): 0.0  
 $f_A$  (mi/h): 1.3  
 $f_M$  (mi/h): 1.6  
 FFS (mi/h): 57.2

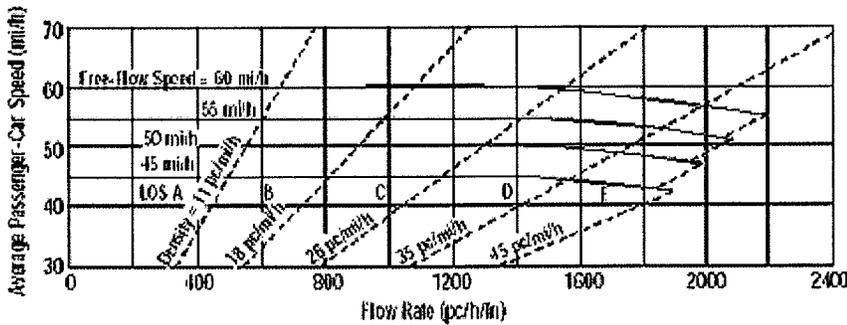
### Operations

Operational (LOS)  
 Flow Rate,  $v_p$  (pc/h/ln): 755  
 Speed, S (mi/h): 57.2  
 D (pc/mi/ln): 13.2  
 LOS: B

### Design

Design (N)  
 Required Number of Lanes, N  
 Flow Rate,  $v_p$  (pc/h)  
 Max Service Flow Rate (pc/h/ln)  
 Design LOS

## MULTILANE HIGHWAYS WORKSHEET(Direction 1)



Application	Input	Output
Operational (LOS)	FFS, N, $v_p$	LOS, S, D
Design (N)	FFS, LOS, $v_p$	N, S, D
Design ( $v_p$ )	FFS, LOS, N	$v_p$ , S, D
Planning (LOS)	FFS, N, AADT	LOS, S, D
Planning (N)	FFS, LOS, AADT	N, S, D
Planning ( $v_p$ )	FFS, LOS, N	$v_p$ , S, D

General Information	Site Information
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Analyst: MKL	Highway/Direction to Travel: Foresthill Road - Mitigated
Agency or Company: MRO Engineers, Inc.	From/To: Todd Valley W to Owl Hill Ct
Date Performed: 4/3/2007	Jurisdiction: Placer County
Analysis Time Period: PM Peak Hour	Analysis Year: Buildout Without Forest Ranch

Project Description: Foresthill Divide Community Plan

Oper.(LOS)
  Des. (N)
  Plan. (vp)

### Flow Inputs

Volume, V (veh/h): 956	Peak-Hour Factor, PHF: 0.85
AAADT(veh/h)	%Trucks and Buses, $P_T$ : 2
Peak-Hour Prop of AAADT (veh/d)	%RVs, $P_R$ : 2
Peak-Hour Direction Prop, D	General Terrain: Grade
DDHV (veh/h)	Grade Length (mi): 1.20
Driver Type Adjustment: 1.00	Up/Down %: 3.00
	Number of Lanes: 2

### Calculate Flow Adjustments

$f_p$ : 1.00	$E_R$ : 3.0
$E_T$ : 2.5	$f_{HV}$ : 0.935

Speed Inputs	Calc Speed Adj and FFS
--------------	------------------------

Lane Width, LW (ft): 12.0	$f_{LW}$ (mi/h): 0.0
Total Lateral Clearance, LC (ft): 12.0	$f_{LC}$ (mi/h): 0.0
Access Points, A (A/mi): 3	$f_A$ (mi/h): 0.8
Median Type, M: Undivided	$f_M$ (mi/h): 1.6
FFS (measured)	FFS (mi/h): 57.7
Base Free-Flow Speed, BFFS: 60.0	

Operations	Design
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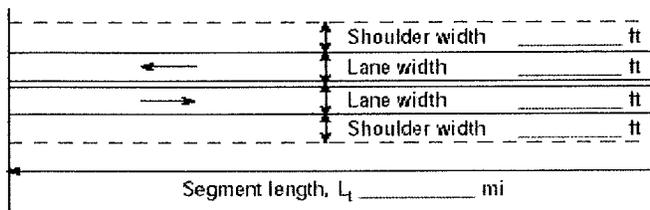
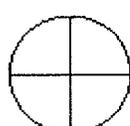
Operational (LOS)	Design (N)
Flow Rate, $v_p$ (pc/h/ln): 601	Required Number of Lanes, N
Speed, S (mi/h): 57.7	Flow Rate, $v_p$ (pc/h)
D (pc/mi/ln): 10.4	Max Service Flow Rate (pc/h/ln)
LOS: A	Design LOS

## DIRECTIONAL TWO-LANE HIGHWAY SEGMENT WORKSHEET

General Information		Site Information	
Analyst	MKL	Highway / Direction of Travel	Foresthill Road - WB - Mitig.
Agency or Company	MRO Engineers, Inc.	From/To	Todd Valley W to Owl Hill Ct.
Date Performed	2/27/07	Jurisdiction	Placer County
Analysis Time Period	PM Peak Hour	Analysis Year	Buildout Without Forest Ranch

Project Description: *Foresthill Divide Community Plan*

### Input Data

 <p style="margin-top: 10px;">Analysis direction vol., <math>V_d</math>      1119veh/h          Opposing direction vol., <math>V_o</math>      956veh/h</p>	<div style="display: flex; justify-content: space-between;"> <div style="text-align: center;">  <p>Show North Arrow</p> </div> <div> <input checked="" type="checkbox"/> Class I highway    <input type="checkbox"/> Class II highway                  Terrain    <input type="checkbox"/> Level    <input type="checkbox"/> Rolling                  Grade Length 1.20 mi    Up/down -3.0                  Peak-hour factor, PHF    0.88                  No-passing zone    100%                  % Trucks and Buses, <math>P_T</math>    4 %                  % Recreational vehicles, <math>P_R</math>    2 %                  Access points/ mi    4             </div> </div>
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Average Travel Speed		Analysis Direction (d)	Opposing Direction (o)
Passenger-car equivalents for trucks, $E_T$ (Exhibit 20-9 or 20-15)		1.1	3.7
Passenger-car equivalents for RVs, $E_R$ (Exhibit 20-9 or 20-17)		1.0	1.0
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		0.996	0.901
Grade adjustment factor <sup>1</sup> , $f_G$ (Exhibit 20-7 or 20-13)		1.00	1.00
Directional flow rate <sup>2</sup> , $v_i(\text{pc/h})=V_i/(\text{PHF} \cdot f_{HV} \cdot f_G)$		1277	1205
Free-Flow Speed from Field Measurement		Estimated Free-Flow Speed	
Field measured speed <sup>3</sup> , $S_{FM}$ mi/h		Base free-flow speed <sup>3</sup> , $\text{BFFS}_{FM}$ 60.0 mi/h	
Observed volume <sup>3</sup> , $V_f$ veh/h		Adj. for lane width and shoulder width <sup>3</sup> , $f_{LS}$ (Exh 20-5)      0.0 mi/h	
Free-flow speed, $\text{FFS}_d = S_{FM} + 0.00776(V_f / f_{HV})$ mi/h		Adj. for access points <sup>3</sup> , $f_A$ (Exhibit 20-5)      1.0 mi/h	
Adjustment for no-passing zones, $f_{np}$ (Exhibit 20-19)      1.1 mi/h		Free-flow speed, $\text{FFS}_d$ ( $\text{FSS}=\text{BFFS}-f_{LS}-f_A$ )      59.0 mi/h	
		Average travel speed, $\text{ATS}=\text{FFS}-0.00776v_p \cdot f_{np}$ 38.7 mi/h	

Percent Time-Spent-Following		Analysis Direction (d)	Opposing Direction (o)
Passenger-car equivalents for trucks, $E_T$ (Exhibit 20-10 or 20-16)		1.0	1.0
Passenger-car equivalents for RVs, $E_R$ (Exhibit 20-10 or 20-16)		1.0	1.0
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$		1.000	1.000
Grade adjustment factor <sup>1</sup> , $f_G$ (Exhibit 20-8 or 20-14)		1.00	0.93
Directional flow rate <sup>2</sup> , $v_i(\text{pc/h})=V_i/(\text{PHF} \cdot f_{HV} \cdot f_G)$		1272	1163
Base percent time-spent-following <sup>4</sup> , $\text{BPTSF}(\%)=100(1-e^{-av_d^b})$			85.7
Adj. for no-passing zone, $f_{np}$ (Exhibit 20-20)			12.5
Percent time-spent-following, $\text{PTSF}(\%)=\text{BPTSF}+f_{np}$			92.2

Level of Service and Other Performance Measures		
Level of service, LOS (Exhibit 20-3 or 20-4)		E
Volume to capacity ratio, $v/c=V_p/1,700$		0.75
Peak 15-min veh-miles of travel, $\text{VMT}_{15}(\text{veh} \cdot \text{mi})=0.25L_t(V/\text{PHF})$		381
Peak-hour vehicle-miles of travel, $\text{VMT}_{60}(\text{veh} \cdot \text{mi})=V \cdot L_t$		1343
Peak 15-min total travel time, $\text{TT}_{15}(\text{veh} \cdot \text{h})=\text{VMT}_{15}/\text{ATS}$		9.9

- Notes**
1. If the highway is extended segment (level) or rolling terrain,  $f_G=1.0$ .
  2. If  $v_i(v_d \text{ or } v_o) >=1,700 \text{ pc/h}$ , terminate analysis--the LOS is F.
  3. For the analysis direction only.
  4. Exhibit 20-21 provides factors a and b.
  5. Use alternative Equation 20-14 if some trucks operate at crawl speeds on a specific downgrade.

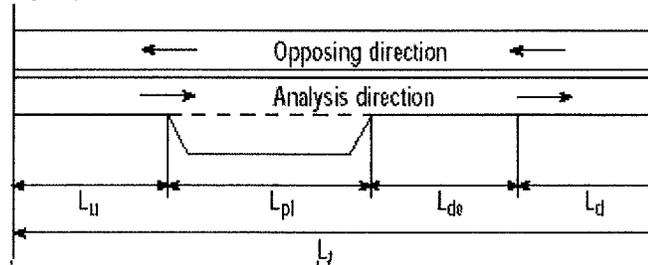
## DIRECTIONAL TWO-LANE HIGHWAY SEGMENT WORKSHEET WITH PASSING LANE WORKSHEET

General Information		Site Information	
Analyst	MKL	Highway of Travel	Foresthill Road - WB - Mitig.
Agency or Company	MRO Engineers, Inc.	From/To	Todd Valley W to Owl Hill Ct.
Date Performed	2/27/07	Jurisdiction	Placer County
Analysis Time Period	PM Peak Hour	Analysis Year	Buildout Without Forest Ranch

Project Description: Foresthill Divide Community Plan

### Input Data

Class I highway     Class II highway



Total length of analysis segment, $L_t$ (mi)	1.2
Length of two-lane highway upstream of the passing lane, $L_u$ (mi)	0.2
Length of passing lane including tapers, $L_{pl}$ (mi)	0.9
Average travel speed, $ATS_d$ (from Directional Two-Lane Highway Segment Worksheet)	38.7
Percent time-spent-following, $PTSF_d$ (from Directional Two-Lane Highway Segment Worksheet)	92.2
Level of service <sup>1</sup> , $LOS_d$ (from Directional Two-Lane Highway Segment Worksheet)	E

### Average Travel Speed

Downstream length of two-lane highway within effective length of passing lane for average travel speed, $L_{de}$ (mi) (Exhibit 20-23)	1.70
Length of two-lane highway downstream of effective length of the passing lane for avg travel speed, $L_d$ (mi) $L_d = L_t - (L_u + L_{pl} + L_{de})$	-1.60
Adj. factor for the effect of passing lane on average speed, $f_{pl}$ (Exhibit 20-24)	1.11
Average travel speed including passing lane <sup>2</sup> , $ATS_{pl} = (ATS_d * L_t) / (L_u + L_d + (L_{pl}/f_{pl}) + (2L_{de}/(1+f_{pl})))$	42.1

### Percent Time-Spent-Following

Downstream length of two-lane highway within effective length of passing lane for percent time-spent-following, $L_{de}$ (mi) (Exhibit 20-23)	3.60
Length of two-lane highway downstream of effective length of the passing lane for percent-time-following, $L_d$ (mi) $L_d = L_t - (L_u + L_{pl} + L_{de})$	-3.50
Adj. factor for the effect of passing lane on percent time-spent-following, $f_{pl}$ (Exhibit 20-24)	0.62
Percent time-spent-following including passing lane <sup>3</sup> , $PTSF_{pl}$ (%) $PTSF_{pl} = PTSF_d [L_u + L_d + f_{pl}L_{pl} + ((1+f_{pl})/2)L_{de}] / L_t$	63.1

### Level of Service and Other Performance Measures<sup>4</sup>

Level of service including passing lane $LOS_{pl}$ (Exhibit 20-3 or 20-4)	D
Peak 15-min total travel time, $TT_{15}$ (veh-h) $TT_{15} = VMT_{15}/ATS_{pl}$	9.0

### Notes

1. If  $LOS_d = F$ , passing lane analysis cannot be performed.
2. If  $L_d < 0$ , use alternative Equation 20-22.
3. If  $L_d < 0$ , use alternative Equation 20-20.
4. v/c,  $VMT_{15}$  and  $VMT_{60}$  are calculated on Directional Two-Lane Highway Segment Worksheet.

**APPENDIX J**

**CUMULATIVE + PROJECT CONDITIONS  
BUILDOUT "WITH FOREST RANCH" SCENARIO  
LEVEL OF SERVICE CALCULATION WORKSHEETS**

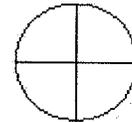
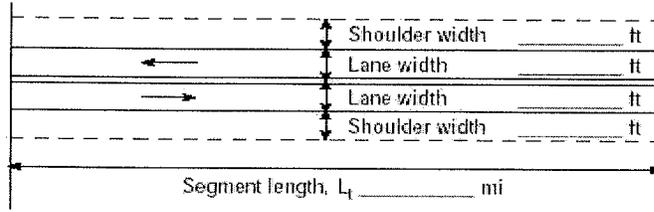


## DIRECTIONAL TWO-LANE HIGHWAY SEGMENT WORKSHEET

General Information		Site Information	
Analyst	MKL	Highway / Direction of Travel	Foresthill Road - Eastbound
Agency or Company	MRO Engineers, Inc.	From/To	Bridge to Spring Garden Road
Date Performed	3/5/07	Jurisdiction	Placer County
Analysis Time Period	AM Peak Hour	Analysis Year	Buildout With Forest Ranch

Project Description: *Foresthill Divide Community Plan*

### Input Data



<input checked="" type="checkbox"/> Class I highway	<input type="checkbox"/> Class II highway
Terrain <input type="checkbox"/> Level <input type="checkbox"/> Rolling	
Grade Length, 4.00 mi	Up/down 3.0
Peak-hour factor, PHF 0.88	
No-passing zone 55%	
% Trucks and Buses, P <sub>T</sub> 6%	
% Recreational vehicles, P <sub>R</sub> 2%	
Access points/ mi 2	

Analysis direction vol., V<sub>d</sub> 1126veh/h  
 Opposing direction vol., V<sub>o</sub> 1853veh/h

### Average Travel Speed

	Analysis Direction (d)	Opposing Direction (o)
Passenger-car equivalents for trucks, E <sub>T</sub> (Exhibit 20-9 or 20-15)	5.7	1.1
Passenger-car equivalents for RVs, E <sub>R</sub> (Exhibit 20-9 or 20-17)	1.0	1.0
Heavy-vehicle adjustment factor, f <sub>HV</sub> =1/(1+P <sub>T</sub> (E <sub>T</sub> -1)+P <sub>R</sub> (E <sub>R</sub> -1))	0.780	0.994
Grade adjustment factor <sup>1</sup> , f <sub>G</sub> (Exhibit 20-7 or 20-13)	0.95	1.00
Directional flow rate <sup>2</sup> , v <sub>i</sub> (pc/h) v <sub>i</sub> =V <sub>i</sub> /(PHF*f <sub>HV</sub> *f <sub>G</sub> )	1727	2118
Free-Flow Speed from Field Measurement	Estimated Free-Flow Speed	
Field measured speed <sup>3</sup> , S <sub>FM</sub> mi/h	Base free-flow speed <sup>3</sup> , BFFS <sub>FM</sub> 60.0 mi/h	
Observed volume <sup>3</sup> , V <sub>f</sub> veh/h	Adj. for lane width and shoulder width <sup>3</sup> , f <sub>LS</sub> (Exh 20-5) 0.0 mi/h	
Free-flow speed, FFS <sub>d</sub> FFS=S <sub>FM</sub> +0.00776(V <sub>f</sub> /f <sub>HV</sub> ) mi/h	Adj. for access points <sup>3</sup> , f <sub>A</sub> (Exhibit 20-5) 0.5 mi/h	
Adjustment for no-passing zones, f <sub>np</sub> (Exhibit 20-19) 0.7 mi/h	Free-flow speed, FFS <sub>d</sub> (FFS=BFFS-f <sub>LS</sub> -f <sub>A</sub> ) 59.5 mi/h	
	Average travel speed, ATS=FFS-0.00776v <sub>p</sub> -f <sub>np</sub> 29.0 mi/h	

### Percent Time-Spent-Following

	Analysis Direction (d)	Opposing Direction (o)
Passenger-car equivalents for trucks, E <sub>T</sub> (Exhibit 20-10 or 20-16)	1.0	1.0
Passenger-car equivalents for RVs, E <sub>R</sub> (Exhibit 20-10 or 20-16)	1.0	1.0
Heavy-vehicle adjustment factor, f <sub>HV</sub> =1/(1+P <sub>T</sub> (E <sub>T</sub> -1)+P <sub>R</sub> (E <sub>R</sub> -1))	1.000	1.000
Grade adjustment factor <sup>1</sup> , f <sub>G</sub> (Exhibit 20-8 or 20-14)	0.97	1.00
Directional flow rate <sup>2</sup> , v <sub>i</sub> (pc/h)=V <sub>i</sub> /(PHF*f <sub>HV</sub> *f <sub>G</sub> )	1319	2106
Base percent time-spent-following <sup>4</sup> , BPTSF(%)=100(1-e <sup>-av<sub>d</sub><sup>b</sup></sup> )	88.9	
Adj. for no-passing zone, f <sub>np</sub> (Exhibit 20-20)	45.3	
Percent time-spent-following, PTSF(%)=BPTSF+f <sub>np</sub>	106.3	

### Level of Service and Other Performance Measures

Level of service, LOS (Exhibit 20-3 or 20-4)	F
Volume to capacity ratio, v/c=V <sub>p</sub> /1,700	1.02
Peak 15-min veh-miles of travel, VMT <sub>15</sub> (veh-mi)=0.25L <sub>t</sub> (V/PHF)	3455
Peak-hour vehicle-miles of travel, VMT <sub>60</sub> (veh-mi)=V*L <sub>t</sub>	12161
Peak 15-min total travel time, TT <sub>15</sub> (veh-h)=VMT <sub>15</sub> /ATS	119.1

### Notes

1. If the highway is extended segment (level) or rolling terrain, f<sub>G</sub>=1.0.
2. If v<sub>i</sub>(V<sub>d</sub> or V<sub>o</sub>) >=1,700 pc/h, terminate analysis--the LOS is F.
3. For the analysis direction only.
4. Exhibit 20-21 provides factors a and b.
5. Use alternative Equation 20-14 if some trucks operate at crawl speeds on a specific downgrade.

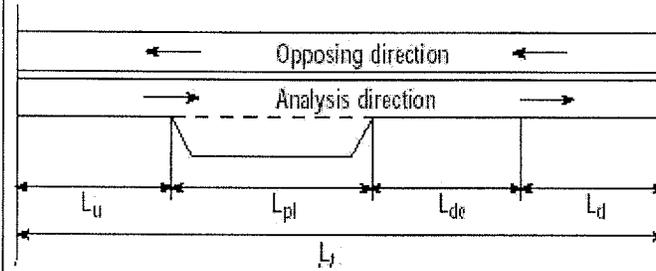
## DIRECTIONAL TWO-LANE HIGHWAY SEGMENT WORKSHEET WITH PASSING LANE WORKSHEET

General Information		Site Information	
Analyst	MKL	Highway of Travel	Foresthill Road - Eastbound
Agency or Company	MRO Engineers, Inc.	From/To	Bridge to Spring Garden Road
Date Performed	3/5/07	Jurisdiction	Placer County
Analysis Time Period	AM Peak Hour	Analysis Year	Buildout With Forest Ranch

Project Description: *Foresthill Divide Community Plan*

### Input Data

Class I highway     Class II highway



Total length of analysis segment, $L_t$ (mi)	10.8
Length of two-lane highway upstream of the passing lane, $L_u$ (mi)	3.0
Length of passing lane including tapers, $L_{pl}$ (mi)	4.9
Average travel speed, $ATS_d$ (from Directional Two-Lane Highway Segment Worksheet)	29.0
Percent time-spent-following, $PTSF_d$ (from Directional Two-Lane Highway Segment Worksheet)	106.3
Level of service <sup>1</sup> , $LOS_d$ (from Directional Two-Lane Highway Segment Worksheet)	F

### Average Travel Speed

Downstream length of two-lane highway within effective length of passing lane for average travel speed, $L_{de}$ (mi) (Exhibit 20-23)	1.70
Length of two-lane highway downstream of effective length of the passing lane for avg travel speed, $L_d$ (mi) $L_d = L_t - (L_u + L_{pl} + L_{de})$	1.20
Adj. factor for the effect of passing lane on average speed, $f_{pl}$ (Exhibit 20-24)	1.11
Average travel speed including passing lane <sup>2</sup> , $ATS_{pl} = (ATS_d * L_t) / (L_u + L_d + (L_{pl}/f_{pl}) + (2L_{de}/(1+f_{pl})))$	30.6

### Percent Time-Spent-Following

Downstream length of two-lane highway within effective length of passing lane for percent time-spent-following, $L_{de}$ (mi) (Exhibit 20-23)	3.60
Length of two-lane highway downstream of effective length of the passing lane for percent-time-following, $L_d$ (mi) $L_d = L_t - (L_u + L_{pl} + L_{de})$	-0.70
Adj. factor for the effect of passing lane on percent time-spent-following, $f_{pl}$ (Exhibit 20-24)	0.62
Percent time-spent-following including passing lane <sup>3</sup> , $PTSF_{pl}(\%)$ $PTSF_{pl} = PTSF_d [L_u + L_d + f_{pl}L_{pl} + ((1+f_{pl})/2)L_{de}] / L_t$	81.5

### Level of Service and Other Performance Measures<sup>4</sup>

Level of service including passing lane $LOS_{pl}$ (Exhibit 20-3 or 20-4)	E
Peak 15-min total travel time, $TT_{15}(\text{veh-h})$ $TT_{15} = VMT_{15}/ATS_{pl}$	112.8

### Notes

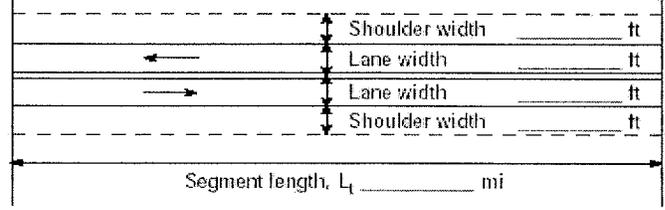
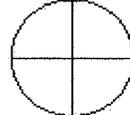
1. If  $LOS_d = F$ , passing lane analysis cannot be performed.
2. If  $L_d < 0$ , use alternative Equation 20-22.
3. If  $L_d < 0$ , use alternative Equation 20-20.
4. v/c,  $VMT_{15}$  and  $VMT_{60}$  are calculated on Directional Two-Lane Highway Segment Worksheet.

## DIRECTIONAL TWO-LANE HIGHWAY SEGMENT WORKSHEET

General Information		Site Information	
Analyst	MKL	Highway / Direction of Travel	Foresthill Road - Westbound
Agency or Company	MRO Engineers, Inc.	From/To	Bridge to Spring Garden Road
Date Performed	3/5/07	Jurisdiction	Placer County
Analysis Time Period	AM Peak Hour	Analysis Year	Buildout With Forest Ranch

Project Description: *Foresthill Divide Community Plan*

### Input Data

 <p style="margin-left: 20px;">Shoulder width _____ ft</p> <p style="margin-left: 20px;">Lane width _____ ft</p> <p style="margin-left: 20px;">Lane width _____ ft</p> <p style="margin-left: 20px;">Shoulder width _____ ft</p> <p style="margin-left: 20px;">Segment length, <math>L_1</math> _____ mi</p>	<div style="display: flex; align-items: center;"> <div style="margin-right: 20px;">  <p>Show North Arrow</p> </div> <div> <p><input checked="" type="checkbox"/> Class I highway    <input type="checkbox"/> Class II highway</p> <p>Terrain    <input type="checkbox"/> Level    <input type="checkbox"/> Rolling</p> <p>Grade Length 4.00 mi    Up/down -3.0</p> <p>Peak-hour factor, PHF 0.88</p> <p>No-passing zone 88%</p> <p>% Trucks and Buses, <math>P_T</math> 2%</p> <p>% Recreational vehicles, <math>P_R</math> 2%</p> <p>Access points/ mi 2</p> </div> </div>
<p>Analysis direction vol., <math>V_d</math> 1853veh/h</p> <p>Opposing direction vol., <math>V_o</math> 1126veh/h</p>	

### Average Travel Speed

	Analysis Direction (d)	Opposing Direction (o)
Passenger-car equivalents for trucks, $E_T$ (Exhibit 20-9 or 20-15)	1.1	5.7
Passenger-car equivalents for RVs, $E_R$ (Exhibit 20-9 or 20-17)	1.0	1.0
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$	0.998	0.914
Grade adjustment factor <sup>1</sup> , $f_G$ (Exhibit 20-7 or 20-13)	1.00	0.95
Directional flow rate <sup>2</sup> , $v_i$ (pc/h) $v_i=V_i/(PHF \cdot f_{HV} \cdot f_G)$	2110	1473
Free-Flow Speed from Field Measurement	Estimated Free-Flow Speed	
Field measured speed <sup>3</sup> , $S_{FM}$ mi/h	Base free-flow speed <sup>3</sup> , $BFFS_{FM}$ 60.0 mi/h	
Observed volume <sup>3</sup> , $V_f$ veh/h	Adj. for lane width and shoulder width <sup>3</sup> , $f_{LS}$ (Exh 20-5) 0.0 mi/h	
Free-flow speed, $FFS_d = FFS_{FM} + 0.00776(V_f \cdot f_{HV})$ mi/h	Adj. for access points <sup>3</sup> , $f_A$ (Exhibit 20-5) 0.5 mi/h	
Adjustment for no-passing zones, $f_{np}$ (Exhibit 20-19) 0.8 mi/h	Free-flow speed, $FFS_d$ ( $FSS=BFFS \cdot f_{LS} \cdot f_A$ ) 59.5 mi/h	
	Average travel speed, $ATS=FFS-0.00776v_p \cdot f_{np}$ 30.9 mi/h	

### Percent Time-Spent-Following

	Analysis Direction (d)	Opposing Direction (o)
Passenger-car equivalents for trucks, $E_T$ (Exhibit 20-10 or 20-16)	1.0	1.0
Passenger-car equivalents for RVs, $E_R$ (Exhibit 20-10 or 20-16)	1.0	1.0
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$	1.000	1.000
Grade adjustment factor <sup>1</sup> , $f_G$ (Exhibit 20-8 or 20-14)	1.00	0.97
Directional flow rate <sup>2</sup> , $v_i$ (pc/h) $=V_i/(PHF \cdot f_{HV} \cdot f_G)$	2106	1319
Base percent time-spent-following <sup>4</sup> , $BPTSF(\%)=100(1-e^{-a \cdot v_d^b})$		95.3
Adj. for no-passing zone, $f_{np}$ (Exhibit. 20-20)		48.6
Percent time-spent-following, $PTSF(\%)=BPTSF+f_{np}$		125.2

### Level of Service and Other Performance Measures

Level of service, LOS (Exhibit 20-3 or 20-4)	F
Volume to capacity ratio, $v/c=V_f/1,700$	1.24
Peak 15-min veh-miles of travel, $VMT_{15}$ (veh- mi) $=0.25L_1(V/PHF)$	5685
Peak-hour vehicle-miles of travel, $VMT_{60}$ (veh- mi) $=V \cdot L_1$	20012
Peak 15-min total travel time, $TT_{15}$ (veh-h) $=VMT_{15}/ATS$	183.9

### Notes

1. If the highway is extended segment (level) or rolling terrain,  $f_G=1.0$ .
2. If  $v_i(v_d$  or  $v_o) \geq 1,700$  pc/h, terminate analysis--the LOS is F.
3. For the analysis direction only.
4. Exhibit 20-21 provides factors a and b.
5. Use alternative Equation 20-14 if some trucks operate at crawl speeds on a specific downgrade.

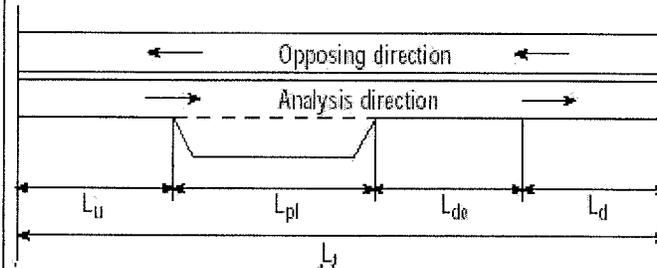
## DIRECTIONAL TWO-LANE HIGHWAY SEGMENT WORKSHEET WITH PASSING LANE WORKSHEET

General Information		Site Information	
Analyst	MKL	Highway of Travel	Foresthill Road - Westbound
Agency or Company	MRO Engineers, Inc.	From/To	Bridge to Spring Garden Road
Date Performed	3/5/07	Jurisdiction	Placer County
Analysis Time Period	AM Peak Hour	Analysis Year	Buildout With Forest Ranch

Project Description: Foresthill Divide Community Plan

### Input Data

Class I highway     Class II highway



Total length of analysis segment, $L_t$ (mi)	10.8
Length of two-lane highway upstream of the passing lane, $L_u$ (mi)	4.8
Length of passing lane including tapers, $L_{pl}$ (mi)	1.3
Average travel speed, $ATS_d$ (from Directional Two-Lane Highway Segment Worksheet)	30.9
Percent time-spent-following, $PTSF_d$ (from Directional Two-Lane Highway Segment Worksheet)	125.2
Level of service <sup>1</sup> , $LOS_d$ (from Directional Two-Lane Highway Segment Worksheet)	F

### Average Travel Speed

Downstream length of two-lane highway within effective length of passing lane for average travel speed, $L_{de}$ (mi) (Exhibit 20-23)	1.70
Length of two-lane highway downstream of effective length of the passing lane for avg travel speed, $L_d$ (mi) $L_d = L_t - (L_u + L_{pl} + L_{de})$	3.00
Adj. factor for the effect of passing lane on average speed, $f_{pl}$ (Exhibit 20-24)	1.11
Average travel speed including passing lane <sup>2</sup> , $ATS_{pl} = (ATS_d * L_t) / (L_u + L_d + (L_{pl}/f_{pl}) + (2L_{de}/(1+f_{pl})))$	31.5

### Percent Time-Spent-Following

Downstream length of two-lane highway within effective length of passing lane for percent time-spent-following, $L_{de}$ (mi) (Exhibit 20-23)	3.60
Length of two-lane highway downstream of effective length of the passing lane for percent-time-following, $L_d$ (mi) $L_d = L_t - (L_u + L_{pl} + L_{de})$	1.10
Adj. factor for the effect of passing lane on percent time-spent-following, $f_{pl}$ (Exhibit 20-24)	0.62
Percent time-spent-following including passing lane <sup>3</sup> , $PTSF_{pl}$ (%) $PTSF_{pl} = PTSF_d [L_u + L_d + f_{pl}L_{pl} + ((1+f_{pl})/2)L_{de}] / L_t$	111.5

### Level of Service and Other Performance Measures<sup>4</sup>

Level of service including passing lane $LOS_{pl}$ (Exhibit 20-3 or 20-4)	F
Peak 15-min total travel time, $TT_{15}$ (veh-h) $TT_{15} = VMT_{15}/ATS_{pl}$	180.2

### Notes

1. If  $LOS_d = F$ , passing lane analysis cannot be performed.
2. If  $L_d < 0$ , use alternative Equation 20-22.
3. If  $L_d < 0$ , use alternative Equation 20-20.
4. v/c,  $VMT_{15}$  and  $VMT_{60}$  are calculated on Directional Two-Lane Highway Segment Worksheet.

## DIRECTIONAL TWO-LANE HIGHWAY SEGMENT WORKSHEET

General Information		Site Information	
Analyst	MKL	Highway / Direction of Travel	Foresthill Road - Eastbound
Agency or Company	MRO Engineers, Inc.	From/To	Spring Garden to Todd Valley W
Date Performed	3/5/07	Jurisdiction	Placer County
Analysis Time Period	AM Peak Hour	Analysis Year	Buildout With Forest Ranch

Project Description: Foresthill Divide Community Plan

### Input Data

<p style="text-align: center;">Segment length, <math>L_1</math> _____ mi</p>	<div style="display: flex; justify-content: space-between;"> <div style="text-align: center;"> <p>Show North Arrow</p> </div> <div> <input checked="" type="checkbox"/> Class I highway    <input type="checkbox"/> Class II highway                  Terrain    <input type="checkbox"/> Level    <input type="checkbox"/> Rolling                  Grade Length 1.70 mi    Up/down 3.4                  Peak-hour factor, PHF 0.75                  No-passing zone 29%                  % Trucks and Buses, <math>P_T</math> 3%                  % Recreational vehicles, <math>P_R</math> 2%                  Access points/ mi 4             </div> </div>
Analysis direction vol., $V_d$ 1234veh/h	
Opposing direction vol., $V_o$ 1286veh/h	

### Average Travel Speed

	Analysis Direction (d)	Opposing Direction (o)
Passenger-car equivalents for trucks, $E_T$ (Exhibit 20-9 or 20-15)	4.3	1.1
Passenger-car equivalents for RVs, $E_R$ (Exhibit 20-9 or 20-17)	1.0	1.0
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$	0.909	0.997
Grade adjustment factor <sup>1</sup> , $f_G$ (Exhibit 20-7 or 20-13)	1.00	1.00
Directional flow rate <sup>2</sup> , $v_i(\text{pc/h}) = V_i/(PHF \cdot f_{HV} \cdot f_G)$	1810	1720
Free-Flow Speed from Field Measurement	Estimated Free-Flow Speed	
Field measured speed <sup>3</sup> , $S_{FM}$ mi/h	Base free-flow speed <sup>3</sup> , $BFFS_{FM}$ 60.0 mi/h	
Observed volume <sup>3</sup> , $V_i$ veh/h	Adj. for lane width and shoulder width <sup>3</sup> , $f_{LS}$ (Exh 20-5) 0.0 mi/h	
Free-flow speed, $FFS_d = FFS = S_{FM} + 0.00776(V_i / f_{HV})$ mi/h	Adj. for access points <sup>3</sup> , $f_A$ (Exhibit 20-5) 1.0 mi/h	
Adjustment for no-passing zones, $f_{np}$ (Exhibit 20-19) 0.5 mi/h	Free-flow speed, $FFS_d$ ( $FFS = BFFS \cdot f_{LS} \cdot f_A$ ) 59.0 mi/h	
	Average travel speed, $ATS = FFS \cdot 0.00776 v_p \cdot f_{np}$ 31.1 mi/h	

### Percent Time-Spent-Following

	Analysis Direction (d)	Opposing Direction (o)
Passenger-car equivalents for trucks, $E_T$ (Exhibit 20-10 or 20-16)	1.0	1.0
Passenger-car equivalents for RVs, $E_R$ (Exhibit 20-10 or 20-16)	1.0	1.0
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$	1.000	1.000
Grade adjustment factor <sup>1</sup> , $f_G$ (Exhibit 20-8 or 20-14)	0.94	1.00
Directional flow rate <sup>2</sup> , $v_i(\text{pc/h})=V_i/(PHF \cdot f_{HV} \cdot f_G)$	1743	1715
Base percent time-spent-following <sup>4</sup> , $BPTSF(\%)=100(1-e^{-a v_d^b})$		93.7
Adj. for no-passing zone, $f_{np}$ (Exhibit. 20-20)		3.5
Percent time-spent-following, $PTSF(\%)=BPTSF+f_{np}$		95.4

### Level of Service and Other Performance Measures

Level of service, LOS (Exhibit 20-3 or 20-4)	F
Volume to capacity ratio, $v/c = V_i / 1,700$	1.06
Peak 15-min veh.-miles of travel, $VMT_{15}(\text{veh} \cdot \text{mi}) = 0.25 L_1 (V_i / PHF)$	699
Peak-hour vehicle-miles of travel, $VMT_{60}(\text{veh} \cdot \text{mi}) = V \cdot L_1$	2098
Peak 15-min total travel time, $TT_{15}(\text{veh} \cdot \text{h}) = VMT_{15} / ATS$	22.5

### Notes

1. If the highway is extended segment (level) or rolling terrain,  $f_G=1.0$ .
2. If  $v_i(v_d \text{ or } v_o) \geq 1,700$  pc/h, terminate analysis--the LOS is F.
3. For the analysis direction only.
4. Exhibit 20-21 provides factors a and b.
5. Use alternative Equation 20-14 if some trucks operate at crawl speeds on a specific downgrade.

## DIRECTIONAL TWO-LANE HIGHWAY SEGMENT WORKSHEET WITH PASSING LANE WORKSHEET

General Information		Site Information	
Analyst	MKL	Highway of Travel	Foresthill Road - Eastbound
Agency or Company	MRO Engineers, Inc.	From/To	Spring Garden to Todd Valley W
Date Performed	3/5/07	Jurisdiction	Placer County
Analysis Time Period	AM Peak Hour	Analysis Year	Buildout With Forest Ranch

Project Description: *Foresthill Divide Community Plan*

### Input Data

Class I highway     Class II highway

Total length of analysis segment, $L_t$ (mi)	1.7
Length of two-lane highway upstream of the passing lane, $L_u$ (mi)	0.3
Length of passing lane including tapers, $L_{pl}$ (mi)	1.2
Average travel speed, $ATS_d$ (from Directional Two-Lane Highway Segment Worksheet)	31.1
Percent time-spent-following, $PTSF_d$ (from Directional Two-Lane Highway Segment Worksheet)	95.4
Level of service <sup>1</sup> , $LOS_d$ (from Directional Two-Lane Highway Segment Worksheet)	F

### Average Travel Speed

Downstream length of two-lane highway within effective length of passing lane for average travel speed, $L_{de}$ (mi) (Exhibit 20-23)	1.70
Length of two-lane highway downstream of effective length of the passing lane for avg travel speed, $L_d$ (mi) $L_d = L_t - (L_u + L_{pl} + L_{de})$	-1.50
Adj. factor for the effect of passing lane on average speed, $f_{pl}$ (Exhibit 20-24)	1.11
Average travel speed including passing lane <sup>2</sup> , $ATS_{pl} = (ATS_d * L_t) / (L_u + L_d + (L_{pl}/f_{pl}) + (2L_{de}/(1+f_{pl})))$	33.8

### Percent Time-Spent-Following

Downstream length of two-lane highway within effective length of passing lane for percent time-spent-following, $L_{de}$ (mi) (Exhibit 20-23)	3.60
Length of two-lane highway downstream of effective length of the passing lane for percent-time-following, $L_d$ (mi) $L_d = L_t - (L_u + L_{pl} + L_{de})$	-3.40
Adj. factor for the effect of passing lane on percent time-spent-following, $f_{pl}$ (Exhibit 20-24)	0.62
Percent time-spent-following including passing lane <sup>3</sup> , $PTSF_{pl}$ (%) $PTSF_{pl} = PTSF_d [L_u + L_d + f_{pl}L_{pl} + ((1+f_{pl})/2)L_{de}] / L_t$	65.7

### Level of Service and Other Performance Measures<sup>4</sup>

Level of service including passing lane $LOS_{pl}$ (Exhibit 20-3 or 20-4)	E
Peak 15-min total travel time, $TT_{15}$ (veh-h) $TT_{15} = VMT_{15}/ATS_{pl}$	20.7

### Notes

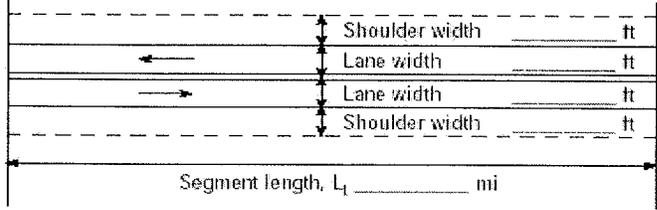
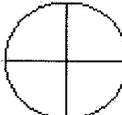
1. If  $LOS_d = F$ , passing lane analysis cannot be performed.
2. If  $L_d < 0$ , use alternative Equation 20-22.
3. If  $L_d < 0$ , use alternative Equation 20-20.
4. v/c,  $VMT_{15}$  and  $VMT_{60}$  are calculated on Directional Two-Lane Highway Segment Worksheet.

## DIRECTIONAL TWO-LANE HIGHWAY SEGMENT WORKSHEET

General Information		Site Information	
Analyst	MKL	Highway / Direction of Travel	Foresthill Road - Westbound
Agency or Company	MRO Engineers, Inc.	From/To	Spring Garden to Todd Valley W
Date Performed	3/5/07	Jurisdiction	Placer County
Analysis Time Period	AM Peak Hour	Analysis Year	Buildout With Forest Ranch

Project Description: Foresthill Divide Community Plan

### Input Data

 <p style="margin-left: 20px;">Shoulder width _____ ft</p> <p style="margin-left: 20px;">Lane width _____ ft</p> <p style="margin-left: 20px;">Lane width _____ ft</p> <p style="margin-left: 20px;">Shoulder width _____ ft</p> <p style="margin-left: 20px;">Segment length, <math>L_1</math> _____ mi</p>	<div style="display: flex; align-items: center;"> <div style="margin-right: 20px;">  <p>Show North Arrow</p> </div> <div> <input checked="" type="checkbox"/> Class I highway    <input type="checkbox"/> Class II highway                  Terrain    <input type="checkbox"/> Level    <input type="checkbox"/> Rolling                  Grade Length 1.70 mi    Up/down -3.4                  Peak-hour factor, PHF 0.88                  No-passing zone 100%                  % Trucks and Buses, <math>P_T</math> 2%                  % Recreational vehicles, <math>P_R</math> 2%                  Access points/ mi 5             </div> </div>
Analysis direction vol., $V_d$ 1286veh/h	
Oposing direction vol., $V_o$ 1234veh/h	

### Average Travel Speed

	Analysis Direction (d)	Oposing Direction (o)
Passenger-car equivalents for trucks, $E_T$ (Exhibit 20-9 or 20-15)	1.1	4.3
Passenger-car equivalents for RVs, $E_R$ (Exhibit 20-9 or 20-17)	1.0	1.0
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$	0.998	0.937
Grade adjustment factor <sup>1</sup> , $f_G$ (Exhibit 20-7 or 20-13)	1.00	1.00
Directional flow rate <sup>2</sup> , $v_i(\text{pc/h}) = V_i / (PHF * f_{HV} * f_G)$	1464	1496
Free-Flow Speed from Field Measurement	Estimated Free-Flow Speed	
Field measured speed <sup>3</sup> , $S_{FM}$ _____ mi/h	Base free-flow speed <sup>3</sup> , $BFFS_{FM}$ 60.0 mi/h	
Observed volume <sup>3</sup> , $V_f$ _____ veh/h	Adj. for lane width and shoulder width <sup>3</sup> , $f_{LS}$ (Exh 20-5) 0.0 mi/h	
Free-flow speed, $FFS_d = S_{FM} + 0.00776(V_f / f_{HV})$ _____ mi/h	Adj. for access points <sup>3</sup> , $f_A$ (Exhibit 20-5) 1.3 mi/h	
Adjustment for no-passing zones, $f_{np}$ (Exhibit 20-19) 0.8 mi/h	Free-flow speed, $FFS_d$ ( $FSS = BFFS - f_{LS} - f_A$ ) 58.8 mi/h	
	Average travel speed, $ATS = FFS - 0.00776v_p - f_{np}$ 35.0 mi/h	

### Percent Time-Spent-Following

	Analysis Direction (d)	Oposing Direction (o)
Passenger-car equivalents for trucks, $E_T$ (Exhibit 20-10 or 20-16)	1.0	1.0
Passenger-car equivalents for RVs, $E_R$ (Exhibit 20-10 or 20-16)	1.0	1.0
Heavy-vehicle adjustment factor, $f_{HV}=1/(1+P_T(E_T-1)+P_R(E_R-1))$	1.000	1.000
Grade adjustment factor <sup>1</sup> , $f_G$ (Exhibit 20-8 or 20-14)	1.00	0.94
Directional flow rate <sup>2</sup> , $v_i(\text{pc/h}) = V_i / (PHF * f_{HV} * f_G)$	1461	1485
Base percent time-spent-following <sup>4</sup> , $BPTSF(\%) = 100(1 - e^{-a v_d^b})$	90.3	
Adj. for no-passing zone, $f_{np}$ (Exhibit. 20-20)	16.1	
Percent time-spent-following, $PTSF(\%) = BPTSF + f_{np}$	98.3	

### Level of Service and Other Performance Measures

Level of service, LOS (Exhibit 20-3 or 20-4)	E
Volume to capacity ratio, $v/c = V_f / 1,700$	0.86
Peak 15-min veh-miles of travel, $VMT_{15}(\text{veh-mi}) = 0.25L_1(V/PHF)$	621
Peak-hour vehicle-miles of travel, $VMT_{60}(\text{veh-mi}) = V * L_1$	2186
Peak 15-min total travel time, $TT_{15}(\text{veh-h}) = VMT_{15}/ATS$	17.8

### Notes

1. If the highway is extended segment (level) or rolling terrain,  $f_G=1.0$ .
2. If  $v_i(v_d \text{ or } v_o) \geq 1,700 \text{ pc/h}$ , terminate analysis--the LOS is F.
3. For the analysis direction only.
4. Exhibit 20-21 provides factors a and b.
5. Use alternative Equation 20-14 if some trucks operate at crawl speeds on a specific downgrade.

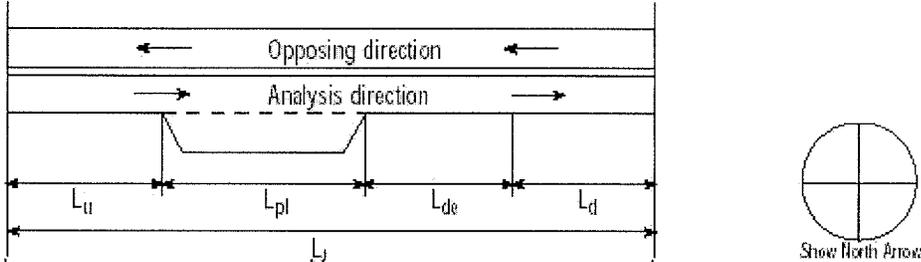
## DIRECTIONAL TWO-LANE HIGHWAY SEGMENT WORKSHEET WITH PASSING LANE WORKSHEET

General Information		Site Information	
Analyst	MKL	Highway of Travel	Foresthill Road - Westbound
Agency or Company	MRO Engineers, Inc.	From/To	Spring Garden to Todd Valley W
Date Performed	3/5/07	Jurisdiction	Placer County
Analysis Time Period	AM Peak Hour	Analysis Year	Buildout With Forest Ranch

Project Description: *Foresthill Divide Community Plan*

### Input Data

Class I highway     Class II highway



Total length of analysis segment, $L_1$ (mi)	1.7
Length of two-lane highway upstream of the passing lane, $L_u$ (mi)	1.7
Length of passing lane including tapers, $L_{pl}$ (mi)	0.0
Average travel speed, $ATS_d$ (from Directional Two-Lane Highway Segment Worksheet)	35.0
Percent time-spent-following, $PTSF_d$ (from Directional Two-Lane Highway Segment Worksheet)	98.3
Level of service <sup>1</sup> , $LOS_d$ (from Directional Two-Lane Highway Segment Worksheet)	E

### Average Travel Speed

Downstream length of two-lane highway within effective length of passing lane for average travel speed, $L_{de}$ (mi) (Exhibit 20-23)	1.70
Length of two-lane highway downstream of effective length of the passing lane for avg travel speed, $L_d$ (mi) $L_d = L_1 - (L_u + L_{pl} + L_{de})$	-1.70
Adj. factor for the effect of passing lane on average speed, $f_{pl}$ (Exhibit 20-24)	1.11
Average travel speed including passing lane <sup>2</sup> , $ATS_{pl} = (ATS_d * L_1) / (L_u + L_d + (L_{pl}/f_{pl}) + (2L_{de}/(1+f_{pl})))$	

### Percent Time-Spent-Following

Downstream length of two-lane highway within effective length of passing lane for percent time-spent-following, $L_{de}$ (mi) (Exhibit 20-23)	3.60
Length of two-lane highway downstream of effective length of the passing lane for percent-time-following, $L_d$ (mi) $L_d = L_1 - (L_u + L_{pl} + L_{de})$	-3.60
Adj. factor for the effect of passing lane on percent time-spent-following, $f_{pl}$ (Exhibit 20-24)	0.62
Percent time-spent-following including passing lane <sup>3</sup> , $PTSF_{pl}(\%)$ $PTSF_{pl} = PTSF_d [L_u + L_d + f_{pl}L_{pl} + ((1+f_{pl})/2)L_{de}] / L_1$	98.3

### Level of Service and Other Performance Measures<sup>4</sup>

Level of service including passing lane $LOS_{pl}$ (Exhibit 20-3 or 20-4)	
Peak 15-min total travel time, $TT_{15}$ (veh-h) $TT_{15} = VMT_{15} / ATS_{pl}$	

### Notes

1. If  $LOS_d = F$ , passing lane analysis cannot be performed.
2. If  $L_d < 0$ , use alternative Equation 20-22.
3. If  $L_d < 0$ , use alternative Equation 20-20.
4. v/c,  $VMT_{15}$  and  $VMT_{60}$  are calculated on Directional Two-Lane Highway Segment Worksheet.